



**Banha University**  
**Faculty of Engineering - Shoubra**  
**Civil Engineering Department**

# **REINFORCED CONCRETE 1 - A**

**For 2<sup>nd</sup> Year Civil – 1<sup>st</sup> Term**

*Prof. Youssef Hashem, Prof. Ahmed Abd-alFattah,  
Assoc. Prof. Fouad Bakheet, Assoc. Prof. Taha Awad & Assoc. Prof. Tarek Sayed*

**Previous Final Examinations**



**R.C. Design Aids** يسمح فقط باستخدام كتاب جداول ومنحنيات التصميم للخرسانة المسلحة •

- Take:  $f_{cu} = 30 \text{ MPa}$ ,  $f_y = 240 \text{ MPa}$  (for  $\Phi \leq 8 \text{ mm}$ ),  $f_y = 360 \text{ MPa}$  (for  $\Phi \geq 10 \text{ mm}$ ),  $\Phi \leq 25 \text{ mm}$ .
- Any missing data may be reasonably assumed according to ECP 2007 & economical design rules.
- Answer all the following questions
- Illustrate your answers with sketches when necessary.
- The exam. consists of one page
- No. of questions: 3
- Total Mark: 60 Marks

**Question (1) [15 Marks] {ILO's: a4, a6, a13, b2, b4, b15, c6, c10}**

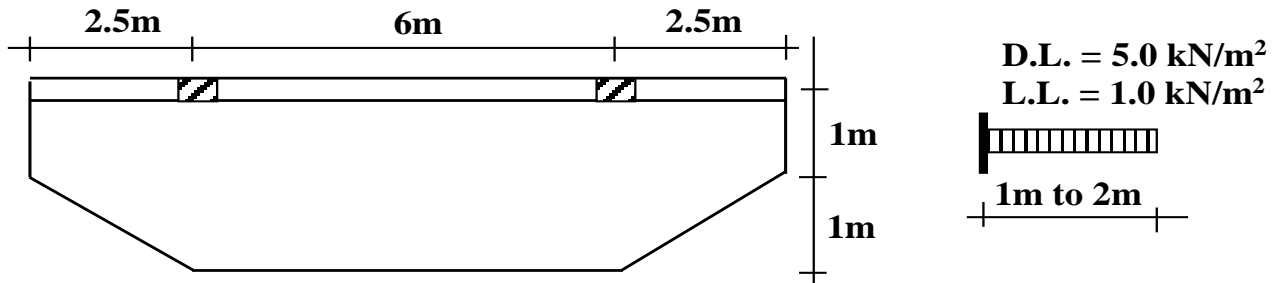
Using the first principles for USLS, calculate  $M_u$  and corresponding **strain ductility** for the following singly-reinforced sections ( $b \times t = 300 \times 900 \text{ mm}$ ,  $t_s = 120 \text{ mm}$ ) of a simply-supported beam ( $L = 6 \text{ m}$ ):

- R-Section ( $a = 2 a_{\min}$ )
- R-Section ( $a = 1.2 a_b$ )
- T-Section ( $a = a_{\min}$ )
- T-Section ( $a = a_{\max}$ )

**Question (2) [27 Marks] {ILO's: a4, a6, a13, b2, b4, b15, c6, c10}**

For the overhanging beam ( $b \times t = 300 \times 800 \text{ mm}$ ,  $t_s = 160 \text{ mm}$ ) in the shown plan of a shed roof under the given slab service dead (D.L.) & live (L.L.) loads and **beam own weight** =  $5 \text{ kN/m'}$ , it is required to:

- Draw the max. ultimate B.M.D., S.F.D. and T.M.D.
- Design & draw critical sections for max. negative B.M & max. positive B.M.
- Design & draw critical sections for shear and torsion as R-sections



**Question (3) [18 Marks] {ILO's: a4, a6, a13, b2, b4, b15, c6, c10}**

- Design and draw a circular column section ( $1.0\% \leq \mu \leq 1.5\%$ ,  $d' = 50 \text{ mm}$ ) to carry a compression load of **4000 kN** for the following eccentricity cases:
  - $e = e_{\min}$  (use spirals)
  - $e = 0.5 \text{ m}$  (use interaction diagrams)
- Design and draw a rectangular column section ( $b = 400 \text{ mm}$ ,  $1.5\% \leq \mu \leq 2.0\%$ ,  $d' = 50 \text{ mm}$ ) to resist a compression force ( $P_{D.L.} = 1200 \text{ kN}$ ,  $P_{L.L.} = 1800 \text{ kN}$ ) and ( $M_{D.L.} = 400 \text{ kN.m}$ ,  $M_{L.L.} = 600 \text{ kN.m}$ ) using the following interaction diagrams:
  - Interaction diagrams with uniform steel arrangement
  - Interaction diagrams with equal top & bottom steel  $A_s = A'_s$

**BOARD OF EXAMINERS:** Professor Ahmed Abdel-Fattah Mahmoud  
Associate Professor Fouad Bakheet Aboud  
Assistant Professor Ahmed Saudi Abdel-Maula  
Assistant Professor Tarek Sayed Mustafa



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- Illustrate your answers with sketches when necessary
- No. of questions : 3
- Total Mark: 60 Marks

**Open Book Examination - Handbook of RC Design Aids is only allowed**

- Take:  $f_{cu} = 30$  MPa,  $f_y = 240$  MPa for  $\Phi \leq 8$  mm,  $f_y = 360$  MPa for  $\Phi \geq 10$  mm,  $\Phi \leq 25$  mm.
- Any missing data may be reasonably assumed according to ECP 2012 & economical design rules.

**Question (1) [15 Marks] {ILO's: a4, a6, a13, b2, b4, b15, c6, c10}**

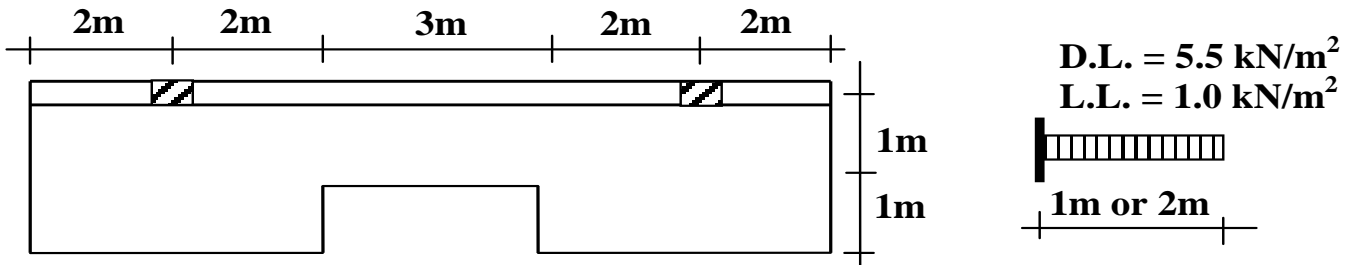
Using the first principles for USLS, calculate  $M_u$  and corresponding **strain ductility** for the following singly-reinforced sections ( $b \times t = 250 \times 700$  mm,  $t_s = 120$  mm) of a continuous beam (interior span  $L = 7$  m):

- T-Section ( $d = d_{min}$ )
- R-Section ( $d = d_{balanced}$ )
- L-Section ( $d = d_{max}$ )
- Trapezoidal section ( $b_{top} = 3b$ ,  $d = d_{min}$ )

**Question (2) [27 Marks] {ILO's: a4, a6, a13, b2, b4, b15, c6, c10}**

For the overhanging beam ( $b \times t = 350 \times 800$  mm,  $t_s = 180$  mm) in the shown plan of a shed roof under the given slab service dead (D.L.) & live (L.L.) loads and **beam own weight** =  $5$  kN/m', it is required to:

- Draw the max. ultimate B.M.D., S.F.D. and T.M.D.
- Design & draw critical sections for max. negative B.M & max. positive B.M.
- Design & draw critical sections for shear and torsion as R-sections



**Question (3) [18 Marks] {ILO's: a4, a6, a13, b2, b4, b15, c6, c10}**

- Calculate the ultimate compression load carried by a circular column section ( $D = 800$  mm) with uniform steel arrangement ( $A_s = 16 \phi 20$ ) for the following eccentricity cases:
  - $e = e_{min}$  (use spirals with  $\phi = 8$  mm,  $p = 50$  mm)
  - $e = 0.25 D$  (use interaction diagrams)
- Design and draw a rectangular column section ( $t = 3 b$ ,  $1.5 \% \leq \mu \leq 2.0 \%$ ,  $d' = 50$  mm) to resist a compression force ( $P_{D.L.} = 1600$  kN,  $P_{L.L.} = 1200$  kN) and ( $M_{D.L.} = 600$  kN.m,  $M_{L.L.} = 400$  kN.m) using the following interaction diagrams:
  - Interaction diagrams with uniform steel arrangement
  - Interaction diagrams with equal top & bottom steel  $A_s = A'_s$

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- Answer all the following questions
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- No. of questions : 3
- Total Mark: 60 Marks

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- Take:  $f_{cu} = 30$  MPa,  $f_y = 360$  MPa for  $\Phi \geq 10$  mm,  $f_y = 240$  MPa for  $\Phi \leq 8$  mm,  $\Phi \leq 25$  mm.
- Any missing data may be reasonably assumed according to ECP 2012 & economical design rules.

**Question (1) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

Using the first principles for USLS, calculate  $M_u$  and corresponding strain ductility for the following singly-reinforced sections ( $b \times t = 300$  mm x 800 mm,  $t_s = 120$  mm):

- T-Section ( $a = a_{min}$ )
- R-Section ( $a = 1.1 a_{balanced}$ )
- L-Section ( $a = a_{max}$ )
- Trapezoidal section ( $b_{top} = 3b$ ,  $a = a_{balanced}$ )

**Question (2) [27 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

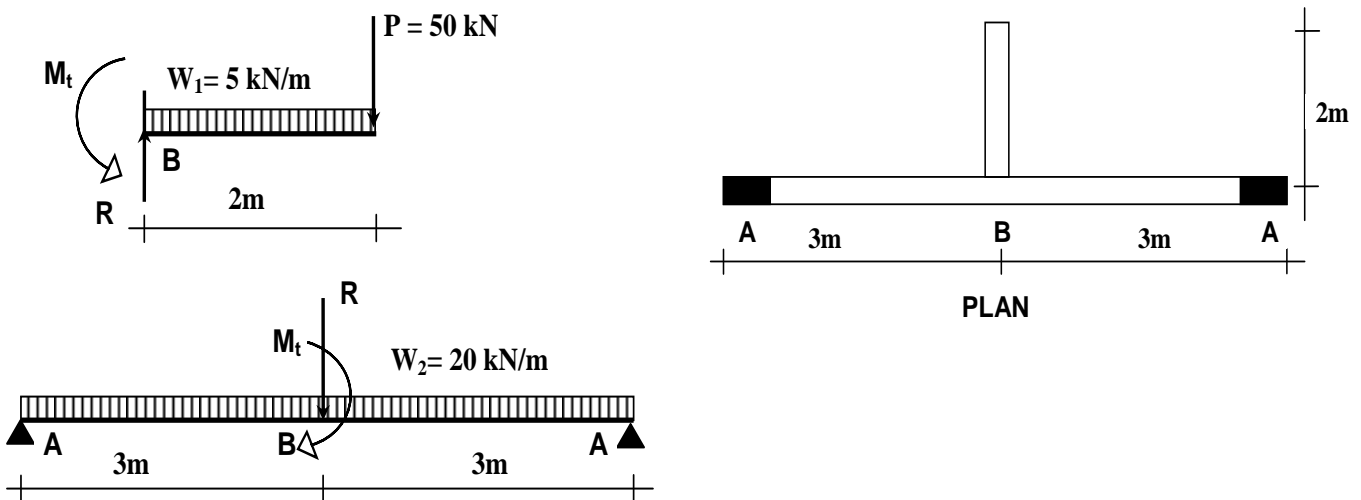
In the shown figure, the simple beam ABA supports a cantilever beam at B under the given service loads.

a- For the cantilever beam (span = 2 m,  $b \times t = 250 \times 600$  mm), it is required to:

- Draw the max. ultimate B.M.D. & S.F.D.
- Design & draw critical sections for bending & shear as R-sections

b- For the simple beam A-B-A (span = 6 m,  $b \times t = 300 \times 700$  mm &  $t_s = 120$  mm), it is required to:

- Draw the max. ultimate B.M.D., S.F.D. & T.M.D.
- Design & draw critical sections for bending as L-sec. & for shear & torsion as R-sec.



**Question (3) [18 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

a. Design & draw a circular column section under an ultimate compression load of 3000 kN for the following eccentricity cases:

- $e = e_{min}$  (use spirals)
- $e = 0.5 D$  (use interaction diagrams)

b. Design & draw a rectangular column section ( $b = 400$  mm,  $1\% \leq \mu \leq 2.0\%$ ,  $d' = 50$  mm) to resist a compression force ( $P_{D.L} = 1200$  kN,  $P_{L.L} = 1800$  kN) and a bending moment ( $M_{D.L} = 500$  kN.m,  $M_{L.L} = 600$  kN.m) using the following interaction diagrams:

- Interaction diagrams with uniform steel arrangement
- Interaction diagrams with equal top & bottom steel  $A_s = A'_s$

**Board of Examiners:** Prof. Dr. Ahmed Abd-El Fattah, Associate Prof. Dr. Fouad Bakheet & Assistant Prof. Dr. Tarek Sayed



- Answer all the following questions
- Illustrate your answers with sketches when necessary
- No. of questions : 3
- Total Mark: 60 Marks

- **Open Book Examination - Handbook of RC Design Aids is only allowed**
- $f_{cu} = 30 \text{ MPa}$ ,  $f_y = 360 \text{ MPa}$  for  $\Phi \geq 10 \text{ mm}$ ,  $f_y = 240 \text{ MPa}$  for  $\Phi = 8 \text{ mm}$ ,  $\Phi \leq 25 \text{ mm}$
- Any missing data may be reasonably assumed according to ECP 2012 & economical design rules.

**Question (1) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

Using the first principles for USLS, calculate  $M_u$  and corresponding **strain ductility** for the following sections (**bxt = 400x600 mm,  $t_s = 140 \text{ mm}$** ) of a cantliver beam of length  $L_c = 2 \text{ m}$ :

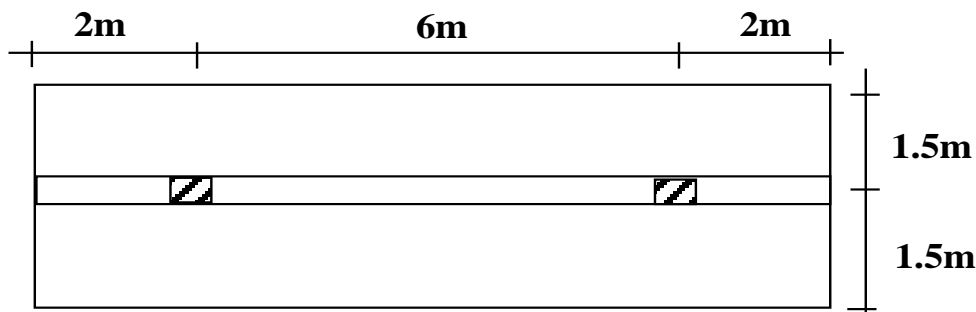
- T-Section ( $A_s = A_{s \text{ min}}$ )
- R-Section ( $C = C_{\text{max}}$ )
- L-Section ( $d = d_{\text{min}}$ )
- R-Section ( $A_s = 4\Phi 25$ ,  $A_s' = 4\Phi 16$ )

**Question (2) [27 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

The shown overhanging beam (**bxt = 300x700 mm,  $t_s = 150 \text{ mm}$** ) supports the slab service dead & live loads of a double-cantilever roof (**D.L. = 5 kN/m<sup>2</sup>, L.L. = 1 kN/m<sup>2</sup>**) and **beam own weight (4.2 kN/m')**.

For the overhanging beam, it is required to:

- Draw the max. ultimate B.M.D.
- Design & draw critical sections for max. negative B.M & max. positive B.M.
- Draw the max. ultimate S.F.D. and T.M.D.
- Design & draw critical sections for shear and torsion as R-sections



**Question (3) [18 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

- Design and draw a **Spiral** circular column to resist ultimate load of  $P_u = 4000 \text{ kN}$  (let  $\mu = 1.0\%$  as a start value)
- Using interaction diagrams, find the ultimate capacity ( $P_u$ ) of the column cross section has the following properties:  $b = 400 \text{ mm}$ ,  $t = 800 \text{ mm}$ ,  $d' = 50 \text{ mm}$ , top and bottom steel arrangement  $A_s = A_s' = 8 \Phi 25$ ,  $e = 2000 \text{ mm}$ .
- Design and draw a rectangular column cross section  $b = 400 \text{ mm}$ ,  $d' = 50 \text{ mm}$ , (let  $t = 1200 \text{ mm}$  as start value) with uniform reinforcement arrangement ( $1.0\% \leq \mu \leq 2.0\%$ ), using column interaction diagrams  $P_{u(\text{comp})} = 5000 \text{ kN}$ ,  $M_u = 1000 \text{ kN.m}$ .
- Design and draw a rectangular column section ( $b = 300 \text{ mm}$ ,  $t = 600 \text{ mm}$ ,  $d' = 50 \text{ mm}$ ) to resist a **tension** force  $T_u = 400 \text{ kN}$  & bending moment  $M_u = 300 \text{ m.kN}$ .



- Answer all the following questions
- Illustrate your answers with sketches when necessary
- No. of questions : 4
- Total Mark: 60 Marks

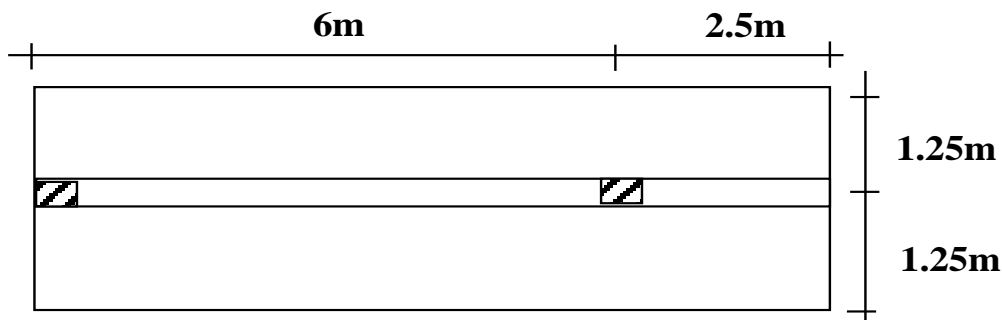
- **Open Book Examination - Handbook of RC Design Aids is only allowed**
- $f_{cu} = 30 \text{ MPa}$ ,  $f_y = 360 \text{ MPa}$  for  $\Phi \geq 10 \text{ mm}$ ,  $f_y = 240 \text{ MPa}$  for  $\Phi = 8 \text{ mm}$ ,  $\Phi \leq 25 \text{ mm}$
- Any missing data may be reasonably assumed according to ECP 2016.

**Question (1) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

The shown overhanging beam ( $b \times t = 300 \times 700 \text{ mm}$ ,  $t_s = 120 \text{ mm}$ ) supports the slab service dead & live loads of a double-cantilever roof (D.L. =  $4 \text{ kN/m}^2$ , L.L. =  $1 \text{ kN/m}^2$ ) and beam own weight ( $4.35 \text{ kN/m}$ ).

For the overhanging beam, it is required to:

- Draw the max. ultimate B.M.D.
- Design & draw critical sections for max. negative B.M & max. positive B.M.



**Question (2) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

For the overhanging beam given in Question (1), it is required to:

- Draw the max. ultimate S.F.D. and T.M.D.
- Design & draw critical sections for shear and torsion as R-sections

**Question (3) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

a- Using the first principles for USLS, calculate  $M_u$  and corresponding **strain ductility** for the following singly-reinforced sections ( $b \times t = 300 \times 800 \text{ mm}$ ,  $t_s = 140 \text{ mm}$ ) of a continuous beam (interior span  $L = 6 \text{ m}$ ):

- T-Section ( $d = d_{\min}$ )
- R-Section ( $d = d_{\text{balanced}}$ )
- L-Section ( $d = d_{\max}$ )

b- Design & draw a **Spiral** circular column to resist ultimate load of  $P_u = 3600 \text{ kN}$  (let  $\mu = 1.0\%$ )

**Question (4) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

- Using interaction diagrams, design & draw a Circular column section ( $D = 700 \text{ mm}$ ,  $d' = 50 \text{ mm}$ ) to resist a compression force  $P_u = 3000 \text{ kN}$  & bending moment  $M_u = 600 \text{ m.kN}$ . ( $1.0\% \leq \mu \leq 2.0\%$ ).
- Design & draw a rectangular section ( $b = 300 \text{ mm}$ ,  $t = 600 \text{ mm}$ ,  $d' = 50 \text{ mm}$ ) to resist a tension force  $T_u = 400 \text{ kN}$  & bending moment  $M_u = 100 \text{ m.kN}$ .
- Design & draw a rectangular column section ( $b = 400 \text{ mm}$ ,  $t = 1000 \text{ mm}$ ,  $d' = 50 \text{ mm}$ ) to resist a compression force  $P_u = 350 \text{ kN}$  & bending moment  $M_u = 1200 \text{ m.kN}$ .

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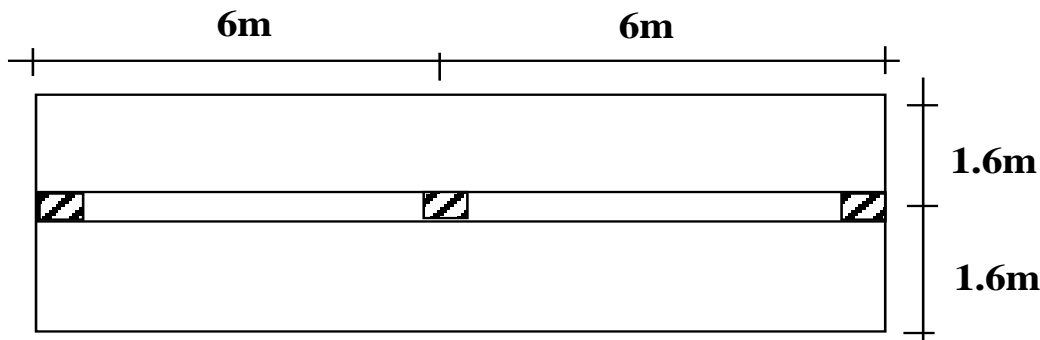
- **Open Book Examination - Handbook of RC Design Aids is only allowed**
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- Any missing data may be reasonably assumed according to ECP 2016.

**Question (1) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

The shown **continuous beam** (bxt = 300x700 mm,  $t_s = 140 \text{ mm}$ ) supports the slab service dead & live loads of a double-cantilever roof (D.L. = 4 kN/m<sup>2</sup>, L.L. = 1 kN/m<sup>2</sup>) and beam own weight (4.35 kN/m').

For the projected continuous beam, it is required to:

- Draw the max. ultimate B.M.D.
- Design & draw critical sections for max. negative B.M & max. positive B.M.



**Question (2) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

For the **continuous beam given in Question (1)**, it is required to:

- Draw the max. ultimate S.F.D. and T.M.D.
- Design & draw critical sections at supports for shear and torsion as R-sections

**Question (3) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

a- Using the first principles for USLS, calculate  $M_u$  and corresponding **strain ductility** for the following singly-reinforced sections (bxt = 250x600 mm,  $t_s = 120 \text{ mm}$ ) of a simply supported beam ( $L = 5 \text{ m}$ ):

- L-Section ( $d = d_{min}$ )
- R-Section ( $d = d_{balanced}$ )
- T-Section ( $d = d_{max}$ )

b- Design & draw a **square** column to resist ultimate load of  $P_u = 5400 \text{ kN}$  (let  $\mu = 1.5\%$ )

**Question (4) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

a- Using interaction diagrams, find the ultimate capacity ( $P_u$ ) of the following column section:  $b = 400 \text{ mm}$ ,  $t = 800 \text{ mm}$ ,  $d' = 50 \text{ mm}$ , top and bottom steel arrangement  $A_s = A_s' = 8 \Phi 25$ ,  $e = 400 \text{ mm}$ .

b- Design & draw a rectangular section ( $b = 300 \text{ mm}$ ,  $t = 1200 \text{ mm}$ ,  $d' = 50 \text{ mm}$ ) to resist a tension force  $T_u = 350 \text{ kN}$  & bending moment  $M_u = 1000 \text{ m.kN}$ .

c- Using column interaction diagrams, design & draw a Circular column section to resist a compression force  $P_u = 5000 \text{ kN}$  & bending moment  $M_u = 500 \text{ m.kN}$ . ( $D = 600 \text{ mm}$ ,  $d' = 50 \text{ mm}$ ,  $1.0\% \leq \mu \leq 2.0\%$ ).



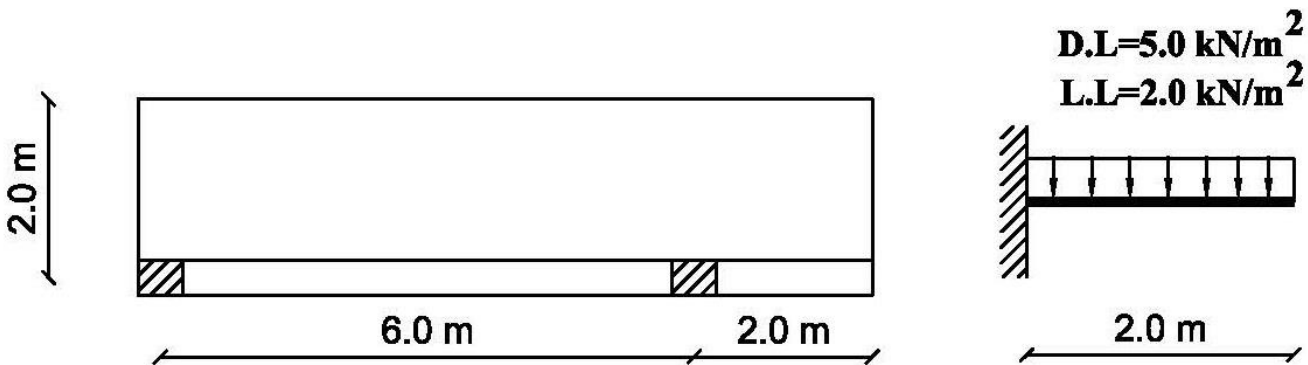
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- No. of questions: 4
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- $f_{cu} = 30 \text{ MPa}$ ,  $f_y = 400 \text{ MPa}$  for  $\Phi \geq 10 \text{ mm}$ ,  $f_y = 240 \text{ MPa}$  for  $\Phi = 8 \text{ mm}$ ,  $\Phi \leq 25 \text{ mm}$
- Any missing data may be reasonably assumed according to ECP 2016.

**Question (1) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

For the shown overhanging beam ( $b \times t = 300 \times 800$ ,  $t_s = 200 \text{ mm}$ ) in the shown plan of a shed roof under the given slab service loads and own weight of beam =  $5.0 \text{ kN/m}$ , it is required to:

- Draw the max. ultimate B.M.D.
- Design & draw the critical sections for max. negative B.M & max. positive B.M.



**Question (2) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

For the overhanging beam given in Question (1), it is required to:

- Draw the max. ultimate S.F.D. and T.M.D.
- Design & draw the critical sections for shear and torsion

**Question (3) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

a- Using the first principles for USLS, calculate  $M_u$  and corresponding **strain ductility** for the following singly-reinforced sections ( $b \times t = 250 \times 700 \text{ mm}$ ,  $t_s = 120 \text{ mm}$ ) of a simply supported beam ( $L = 6 \text{ m}$ ):

- R-Section ( $a = 2 a_{min}$ )
- R-Section ( $a = 1.2 a_b$ )
- T-Section ( $a = a_{max}$ )

b- Design & draw a **Spiral circular** column to resist ultimate load of  $P_u = 4000 \text{ kN}$  (let  $\mu = 1.5\%$ )

**Question (4) [15 Marks] {ILO's: a1, a2, a3, b1, b2, b3, c1, c2}**

a- Using interaction diagrams, design & draw a Circular column section to resist a compression force  $P_u = 4000 \text{ kN}$  & bending moment  $M_u = 700 \text{ m.kN}$ . ( $D = 700 \text{ mm}$ ,  $d' = 50 \text{ mm}$ ,  $1.0\% \leq \mu \leq 2.0\%$ ).

b- Design & draw a rectangular section ( $b = 300 \text{ mm}$ ,  $t = 700 \text{ mm}$ ,  $d' = 50 \text{ mm}$ ) to resist a tension force  $T_u = 600 \text{ kN}$  & bending moment  $M_u = 200 \text{ m.kN}$ .

c- Using interaction diagrams, find the ultimate capacity ( $P_u$ ) of the following column section:  $b = 400 \text{ mm}$ ,  $t = 800 \text{ mm}$ ,  $d' = 50 \text{ mm}$ , top and bottom steel arrangement  $A_s = A_s' = 10 \Phi 25$ ,  $e = 200 \text{ mm}$ .

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